

Mixed FE analysis of viscoelastic cylindrical helixes

Ü. N. Arıbaş and M. H. Omurtag omurtagm@itu.edu.tr

Citation: *AIP Conference Proceedings* **1476**, 61 (2012); doi: 10.1063/1.4751566

View online: <http://dx.doi.org/10.1063/1.4751566>

View Table of Contents: <http://aip.scitation.org/toc/apc/1476/1>

Published by the *American Institute of Physics*

Mixed FE Analysis of Viscoelastic Cylindrical Helixes

Ü.N. Arıbaş¹ and M.H. Omurtag¹

¹Faculty of Civil Engineering, Istanbul Technical University, Istanbul, Turkey, omurtagm@itu.edu.tr

Abstract. In this study, analysis of viscoelastic cylindrical helixes with circular and square cross section is investigated by using the mixed FEM based on Timoshenko beam theory. The Kelvin model is used for the viscoelastic behavior. The analysis is performed in the Laplace domain and the results are transformed back to time domain numerically by Modified Durbin algorithm. The outcome is quite satisfactory besides the necessary engineering precision.

Keywords: Viscoelasticity, Mixed finite element method, Laplace, Cylindrical helix

PACS: 02.70.Dh; 46.35.+z

INTRODUCTION

The materials, such as plastics, natural and synthetic fibers, metals at elevated temperatures, show viscoelastic behavior. In the literature, one of the best known mechanical models for representing viscoelastic material behavior is the Kelvin model. The theoretical foundations for viscoelasticity are well established by many researches, among them, Fung 1965, Malvern 1969, Flügge 1975 and Christensen 1982 can be referred. The Laplace transform approach (Chen 1995, Aköz and Kadioğlu 1999, Temel *et al.* 2004, Argeso *et al.* 2011a, Eratlı *et al.* 2011) have been used in the solution of the various viscoelastic structural elements. In this study, a mixed finite element formulation based on the Timoshenko beam theory is used for dynamic analysis of viscoelastic cylindrical helixes with circular and square cross section. The solutions are carried out in Laplace transform space (LTS). Viscoelastic material behavior is simulated by Kelvin model (see Fig. 1) using correspondence principle. The results are transformed back to time domain numerically by Modified Durbin algorithm (Dubner and Abate 1968, Durbin 1974).

FIELD EQUATIONS AND FUNCTIONAL IN LAPLACE TRANSFORM SPACE

The viscoelastic properties of a body may be accounted for through the use of the correspondence principle (Fung 1965, Malvern 1969). For an isotropic viscoelastic material the Poisson's ratio $\bar{\nu}$ and \bar{E} are transformed material constants in Laplace domain. Explicit form of the viscoelastic material properties exists in Eratlı *et al.* (2011). The field equations based on Timoshenko beam theory and the details about the helix geometry can be found in Omurtag 1990, Omurtag and Aköz 1992 and

Olgun 2004. Variables of the two node element at Frenet coordinates are; displacement vector $\bar{\mathbf{u}} = u_i \mathbf{t} + u_n \mathbf{n} + u_b \mathbf{b}$, the rotational vector $\bar{\boldsymbol{\Omega}}$, force vector $\bar{\mathbf{T}}$ and moment vector $\bar{\mathbf{M}}$, respect to the centroidal axes of the cross section, $\bar{\mathbf{T}}$ and $\bar{\mathbf{M}}$ are the force and the moment vectors in the Laplace space, ρ is the density of material, A is area of the cross section, \mathbf{I} is the moment of inertia, $\bar{\boldsymbol{\gamma}}$ is the unit shear vector, $\bar{\boldsymbol{\kappa}}$ is the unit rotation vector, $\bar{\mathbf{C}}_\gamma$ and $\bar{\mathbf{C}}_\kappa$ are creep compliance matrices. $\bar{\mathbf{q}}$ and $\bar{\mathbf{m}}$ are the distributed external force and moment vectors (Eratlı *et al.* 2011). Hence the corresponding functional in the Laplace domain yields to:

$$\mathbf{I}(\bar{\mathbf{y}}) = \left. \begin{aligned} & -[\bar{\mathbf{u}}, \bar{\mathbf{T}}_{,s}] + [\mathbf{t} \times \bar{\boldsymbol{\Omega}}, \bar{\mathbf{T}}] - [\bar{\mathbf{M}}_{,s}, \bar{\boldsymbol{\Omega}}] - \frac{1}{2}[\bar{\mathbf{C}}_\kappa \bar{\mathbf{M}}, \bar{\mathbf{M}}] - \frac{1}{2}[\bar{\mathbf{C}}_\gamma \bar{\mathbf{T}}, \bar{\mathbf{T}}] + \frac{1}{2} \rho A z^2 [\bar{\mathbf{u}}, \bar{\mathbf{u}}] \\ & + \frac{1}{2} \rho z^2 [\mathbf{I} \bar{\boldsymbol{\Omega}}, \bar{\boldsymbol{\Omega}}] - [\bar{\mathbf{q}}, \bar{\mathbf{u}}] - [\bar{\mathbf{m}}, \bar{\boldsymbol{\Omega}}] + [(\bar{\mathbf{T}} - \hat{\mathbf{T}}), \bar{\mathbf{u}}]_e + [(\bar{\mathbf{M}} - \hat{\mathbf{M}}), \bar{\boldsymbol{\Omega}}]_e + [\hat{\mathbf{u}}, \bar{\mathbf{T}}]_e + [\hat{\boldsymbol{\Omega}}, \bar{\mathbf{M}}]_e \end{aligned} \right\} \quad (1)$$

FINITE ELEMENT FORMULATION

In finite element formulation as stated by Omurtag 1990 and Omurtag and Aköz 1992, linear shape functions ($\phi_i = (\theta_j - \theta) / \Delta\theta$, $\phi_j = (\theta - \theta_i) / \Delta\theta$) are used, where the subscripts represent node numbers of the beam element, $\theta_j > \theta_i$ and $\Delta\theta = (\theta_j - \theta_i)$. This element has two nodes with 2×12 degrees of freedom. $\{u_i, u_n, u_b, \Omega_i, \Omega_n, \Omega_b, T_i, T_n, T_b, M_i, M_n, M_b\}$ is the nodal unknowns of the mixed finite element.

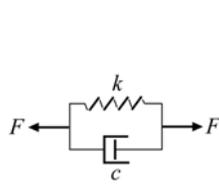


FIGURE 1. Kelvin's model

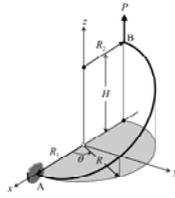


FIGURE 2. Cantilever helical beam

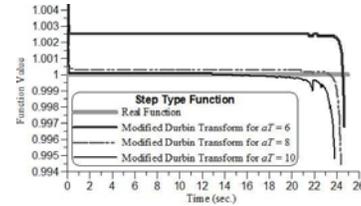


FIGURE 3. Numerical convergence of transformation

NUMERICAL EXAMPLES

Example 1 : Convergence of the Numerical Transformation Algorithm

The inverse Laplace transformation is performed numerically by means of modified Durbin's algorithm. In order to verify the numerical parameter $5 \leq aT \leq 10$ of the numerical algorithm, a numerical convergence test is performed. For this purpose, a known Laplace transformed functions is inverted to the time space numerically for various values of aT and these results are compared by the exact results in order to attain a final decision for the rest of this study. The solutions are performed for the step type loading and the parameters $aT = 6, 8$ and 10 are used in the time interval $0 \leq t \leq 25$ second. As a final conclusion about the parameters of the Durbin's modified inverse Laplace transformation, the error of the result due to the numerical parameter $aT=6$ is negligible compared to $aT=8$ and 10 but the consumed computation

time increases considerably when aT is increased from 6 to 10. Also the numerical behavior of the solution based on $aT=6$ is more stable. Hence in the rest of this study $aT=6$ is used (see Figure 3).

Example 2 : Viscoelastic Helicoidal bar

In the dynamic analysis of viscoelastic helices, a cylindrical helix with circular and square cross-sections is considered (see Figure 2). The viscoelastic material behavior is simulated by using Kelvin's model (see Figure 1). Material and geometrical properties are: the modulus of elasticity $E = 2.06 \times 10^{11}$ N/m², Poisson ratio $\nu = 0.3$, the number of active turns $n = 0.5$, the height of the helix is $H = 300$ cm, the radius of the helix $R_1 = R_2 = 200$ cm, the dimension of square cross-section of helix $a = 12$ cm and the diameter of the circular cross-section $D = 13.54$ cm. The net areas of the square and circular cross-sections are kept constant. The density of material $\rho = 7850$ kg/m³, the damping ratio $f = 0.02$, the intensity of unit step load $P = 10^6$ N. Figure 4 - Figure 7 show time variation u_z at point B and T_z , M_y , M_z at point A, respectively. The maximum peak of vertical displacement at the tip of the helicoidal spring is measured at $t = 0.23$ sec. In the comparison of the displacement u_z of viscoelastic helix with circular and square cross-sections for $t = 0.23$ sec., it is seen that the displacement of the helix with the square cross section is increased by 6% with respect to the circular cross-section. On the other hand, stress resultants are not affected from the change of the cross-section (from circular to square). Results of the square section was verified by other researches existing in the open literature in Erathl *et al.* 2011.

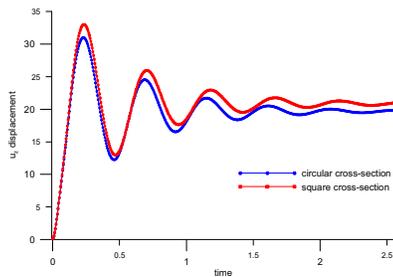


FIGURE 4. The variation of u_z displacement

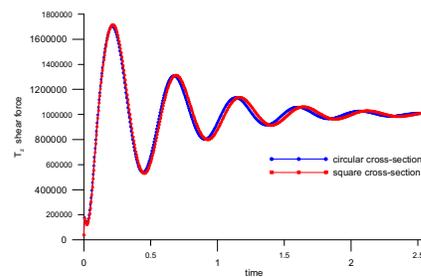


FIGURE 5. The variation of T_z shear force

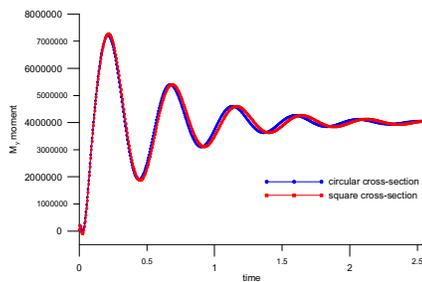


FIGURE 6. The variation of M_y moment

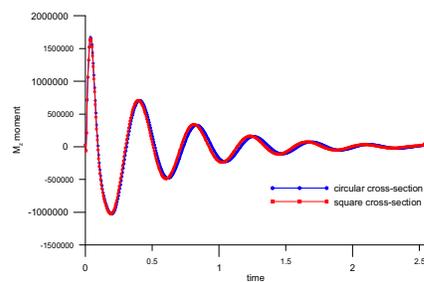


FIGURE 7. The variation of M_z moment

CONCLUSION

A mixed finite element formulation is developed for the dynamic analysis of viscoelastic cylindrical helical beams based on Timoshenko beam theory. The formulation is accomplished in LTS and the viscoelastic properties of a body are accounted using the correspondence principle. The viscoelastic material behavior is simulated by using Kelvin's models. In this study, an example problem is considered with a constant cross-sectional area (circular and square cases are employed) is analyzed with mixed FEM in LTS. The results obtained in transform space are inverted back to time space using Modified Durbin's algorithm and results of these two cross-sections are compared. The obtained results are quite reasonable compared to the existing studies.

ACKNOWLEDGMENTS

This research is supported by Istanbul Technical University Research Fund and this support is gratefully acknowledged by the authors.

REFERENCES

1. R.M. Christensen, *Theory of Viscoelasticity*, New York: Academic Press, 1982.
2. W. Flügge, *Viscoelasticity*, Berlin-Heidelberg: Springer-Verlag, 1975.
3. Y.C. Fung, *Foundations of Solid Mechanics*, Englewood Cliffs-New Jersey: Prentice-Hall, 1965.
4. L.E. Malvern, *Introduction to the Mechanics of Continuous Medium*, Englewood Cliffs-New Jersey: Prentice-Hall, 1969.
5. Y. Aköz and F. Kadioğlu, "The mixed finite element method for the quasi-static and dynamic analysis of viscoelastic Timoshenko beams" in *Int. J. Numer. Meths. Engng.*, edited by Ted Belytschko *et al.*, Vol. 44, 1999, pp. 1909-1932.
6. T.M. Chen, "The hybrid Laplace transform/finite element method applied to the quasi-static and dynamic analysis of viscoelastic Timoshenko beams" in *Int. J. Numer. Meth. Engng.*, edited by Ted Belytschko *et al.*, Vol. 38, 1995, pp. 509-522.
7. H. Dubner and J. Abate, "Numerical inversion of Laplace transforms by relating them to the finite Fourier cosine transform" in *Journal of the Association for Computing Machinery*, edited by Victor Vinau, Vol. 15(1), 1968, pp. 115-123.
8. F. Durbin, "Numerical inversion of Laplace transforms: An efficient improvement to Dubner and Abate's Method" in *Computer Journal*, edited by Professor E. Gelenbe, Vol. 17, 1974, pp. 371-376.
9. M.H. Omurtag and A.Y. Aköz, "The mixed finite element solution of helical beams with variable cross-section under arbitrary loading" in *Comp. and Struct.*, edited by K.J. Bathe, Vol. 43(2), 1992, pp. 325-331.
10. B. Temel, F.F. Çalım, and N. Tütüncü, "Quasi-static and dynamic response of viscoelastic helical rods" in *J. Sound and Vibration*, edited by M.P. Cartmell, Vol. 271, 2004, pp. 921-935.
11. H. Argeso, N. Eratlı, F.F. Çalım, Ü.N. Arıbaş, M.H. Omurtag, "Analysis of viscoelastic conical helixes via mixed finite element method" in *Advances in Applied Mechanics and Modern Information Technology 2011 (AAM&MIT'11)*, Baku, 2011
12. N. Eratlı, F.F. Çalım, Ü.N. Arıbaş ve M.H. Omurtag, "Finite element analysis of conical viscoelastic helixes for various parameters", (in Turkish), in *XVII. National Mechanic Conference (Ulusal Mekanik Kongresi)*, Elazığ, 2011.
13. O. Olgun, "Static and Dynamic analysis of helicoidal bars with mixed finite element method", (in Turkish), M.Sc. Thesis, Istanbul Technical University, 2004.
14. M.H. Omurtag, "Solution of the reinforced cylindrical shells with variable cross-section by using mixed finite element method", (in Turkish), Ph.D. Thesis, Istanbul Technical University, 1990.